

Time and Apparent Time Effects on Compaction & Subsidence

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Time Effects on Compaction & Subsidence

- Creep
- Consolidation

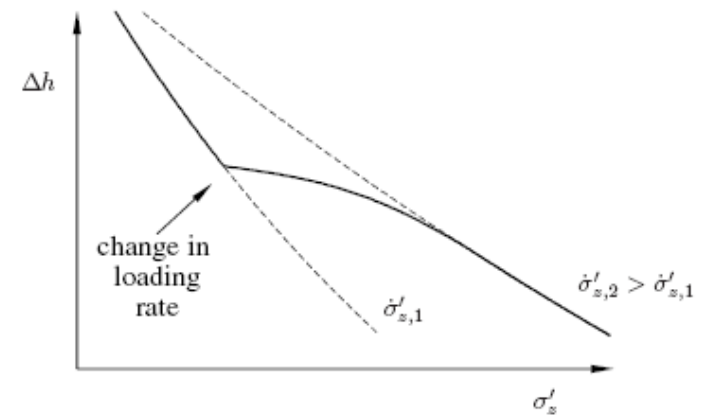
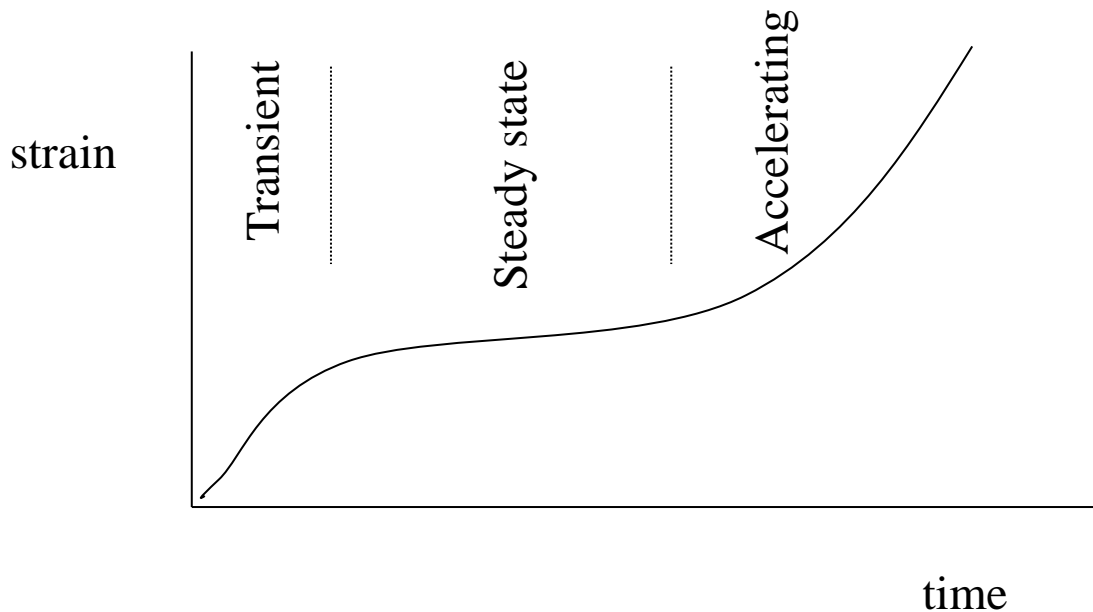
Apparent Time Effects:

- Stress Path / Arching-induced
- Elasto-Plastic Transition

Both Reservoir and Surrounding Rocks are involved

Creep

- Creep is characterized by 3 phases: Transient, Steady-state, and Accelerating, where the rock may undergo failure
- Creep mechanisms are not fully understood – a common assumption is stress-induced corrosion, which is strongly dependent on temperature & the distance in stress space to rock failure



Rate dependent compaction model by de Waal & Smits, 1988

Consolidation

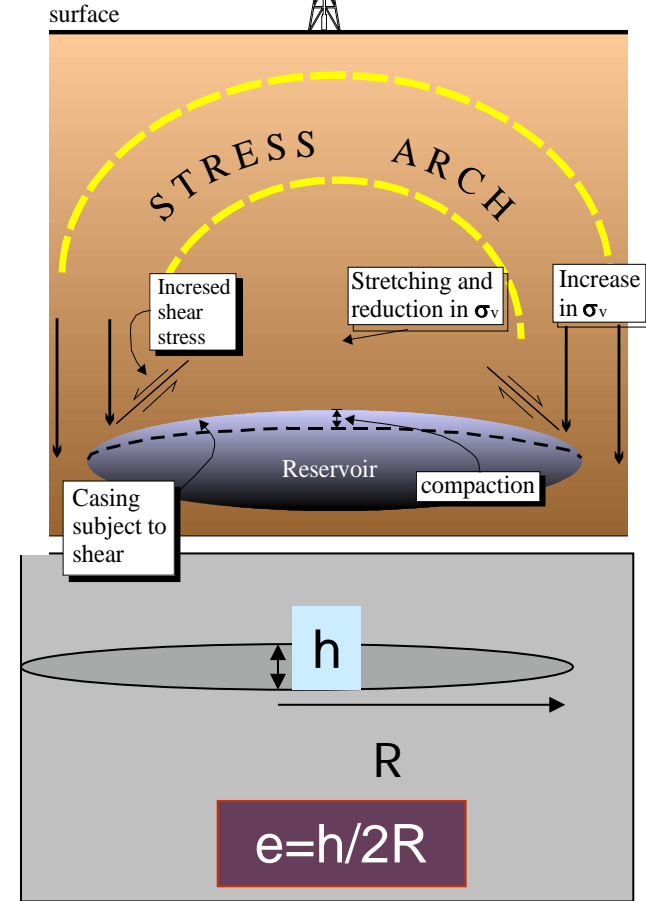
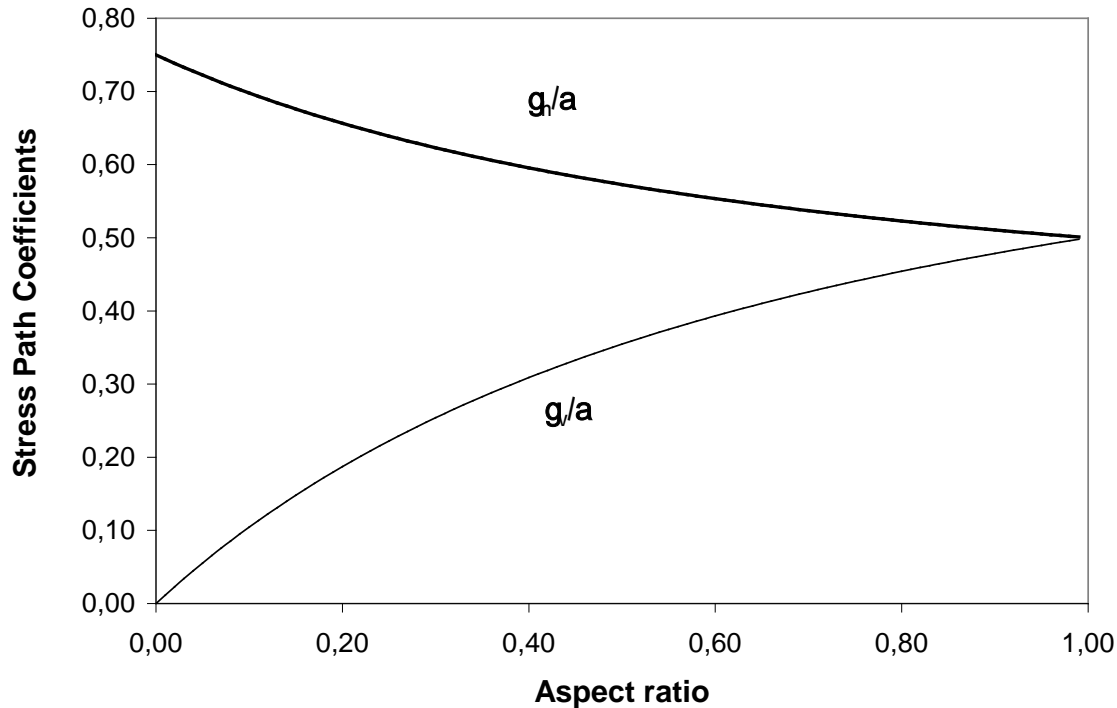
- Pore pressure equilibration in a depleting reservoir occurs at different rates, depending on permeability
- The characteristic time for reaching equilibrium follows a classical diffusion law:

$$t_D \gg \frac{l_D^2}{C_D}$$

$$C_D = \frac{k}{h} \left(M - \frac{C^2}{H} \right) \gg \frac{kK_f}{hf} + \frac{K_f}{f \left(K_{fr} + \frac{4}{3} G_{fr} \right)}$$

- 100m sand with Darcy permeability @ seconds – minutes time-scale
- 10m shale layer with nanoDarcy permeability @ 10-100 years time-scale
- 100m shale with nanoDarcy permeability @ Myears time-scale
- What if all permeabilities are present at all length scales (Mossop's hypothesis)?
- [What about TenBoer?](#)

Stress Path



g_v : "Arching" coefficient = Ds_v/Dp_f

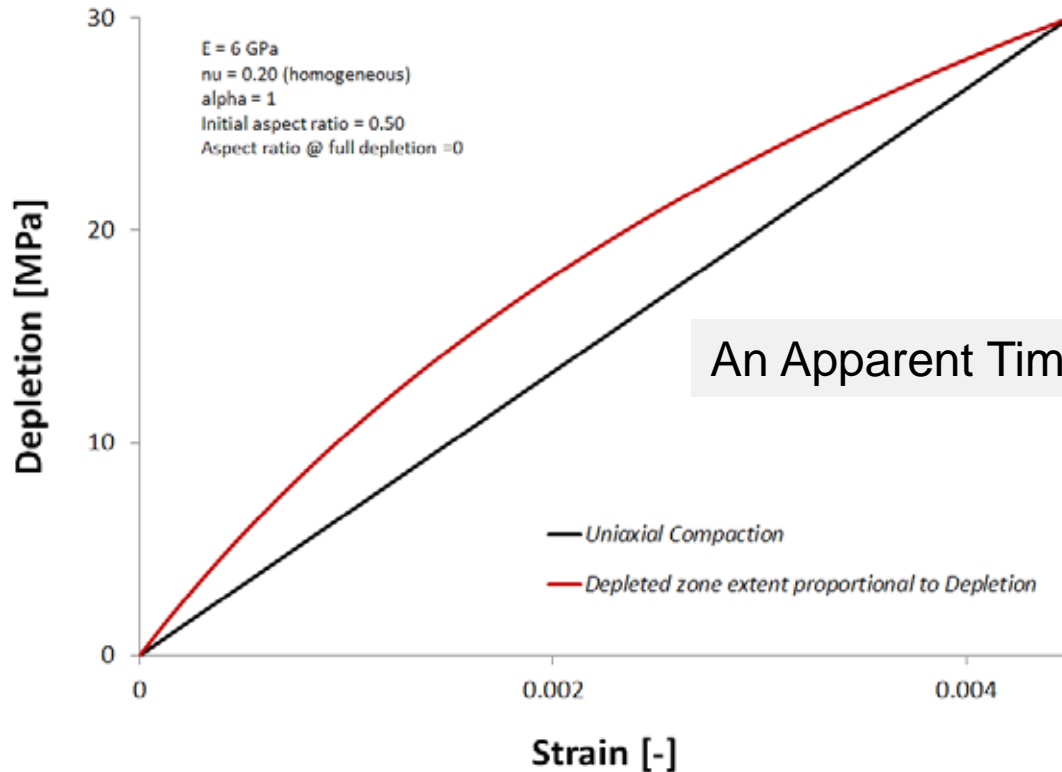
g_h : Describes horizontal stress evolution = Ds_h/Dp_f
(First defined by Schutjens, Hettema a.o.)

Reservoir stress path coefficients from Rudnicki (1999); reservoir is elastically matched to the surroundings (Poisson's ratio = 0.20)

Notice: Stresses (and pore pressure) also change in the surrounding rocks

Reservoir Stress Path: Impact on Compaction

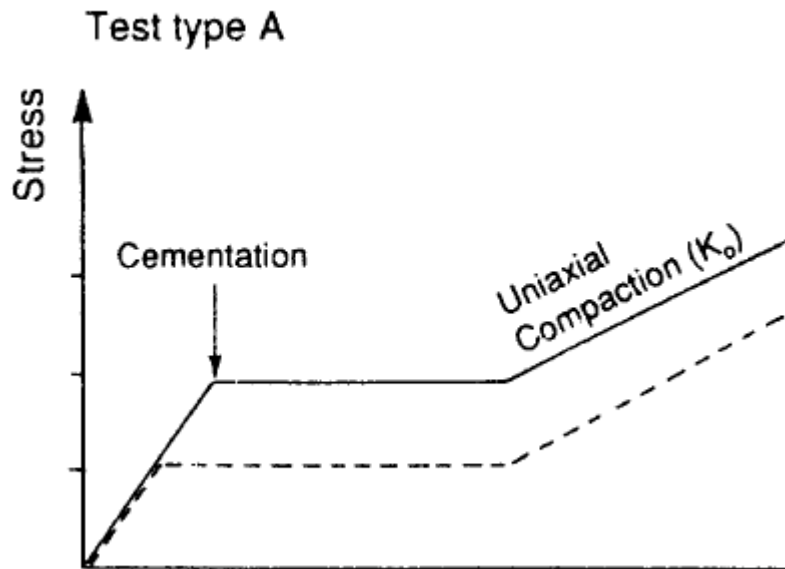
$$\frac{-Dh}{h} = a \frac{(1 - \frac{g_v}{a}) - 2n_{fr} (1 - \frac{g_h}{a})}{E_{fr}} (-Dp_f)$$



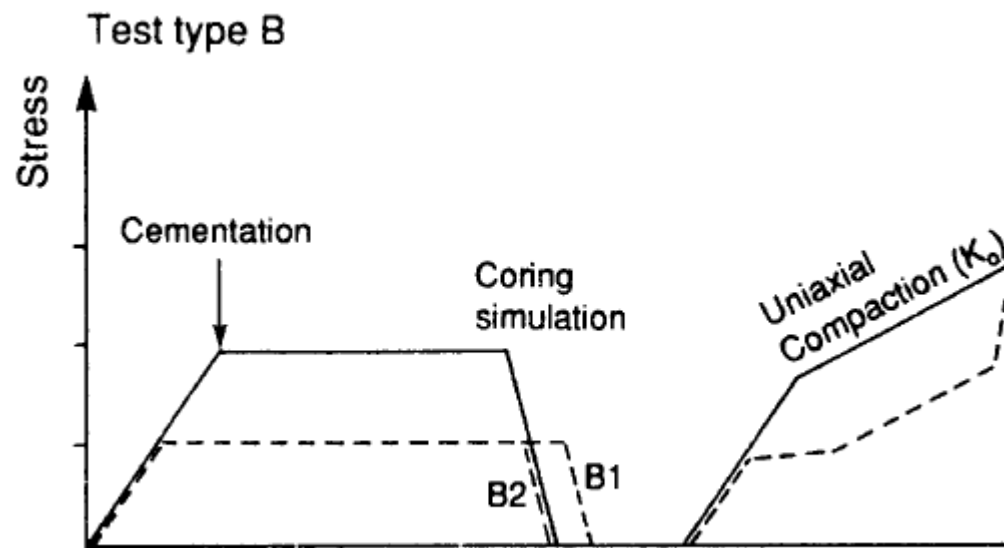
Compaction decreases with increasing reservoir aspect ratio, reflecting enhanced arching

If depleted area increases with depletion (aspect ratio decreases), compaction will accelerate with time

Core vs. Virgin compaction: The "GRONstone" Experience



"Virgin" compaction, along a Reservoir Stress Path

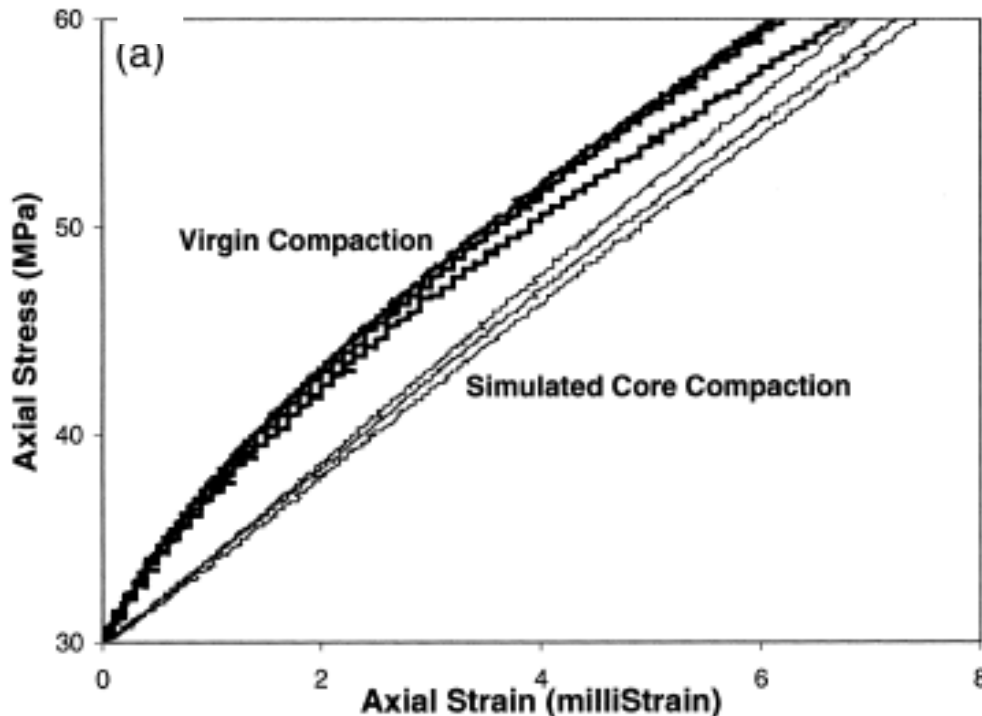


"Coring" simulation, along 2 Coring Stress Paths + "Core" compaction, along a Reservoir Stress Path

Cementation: Sand & e.g. Sodium Silicate + CO_2

Virgin vs. Simulated Core ^{Uniaxial} Compaction

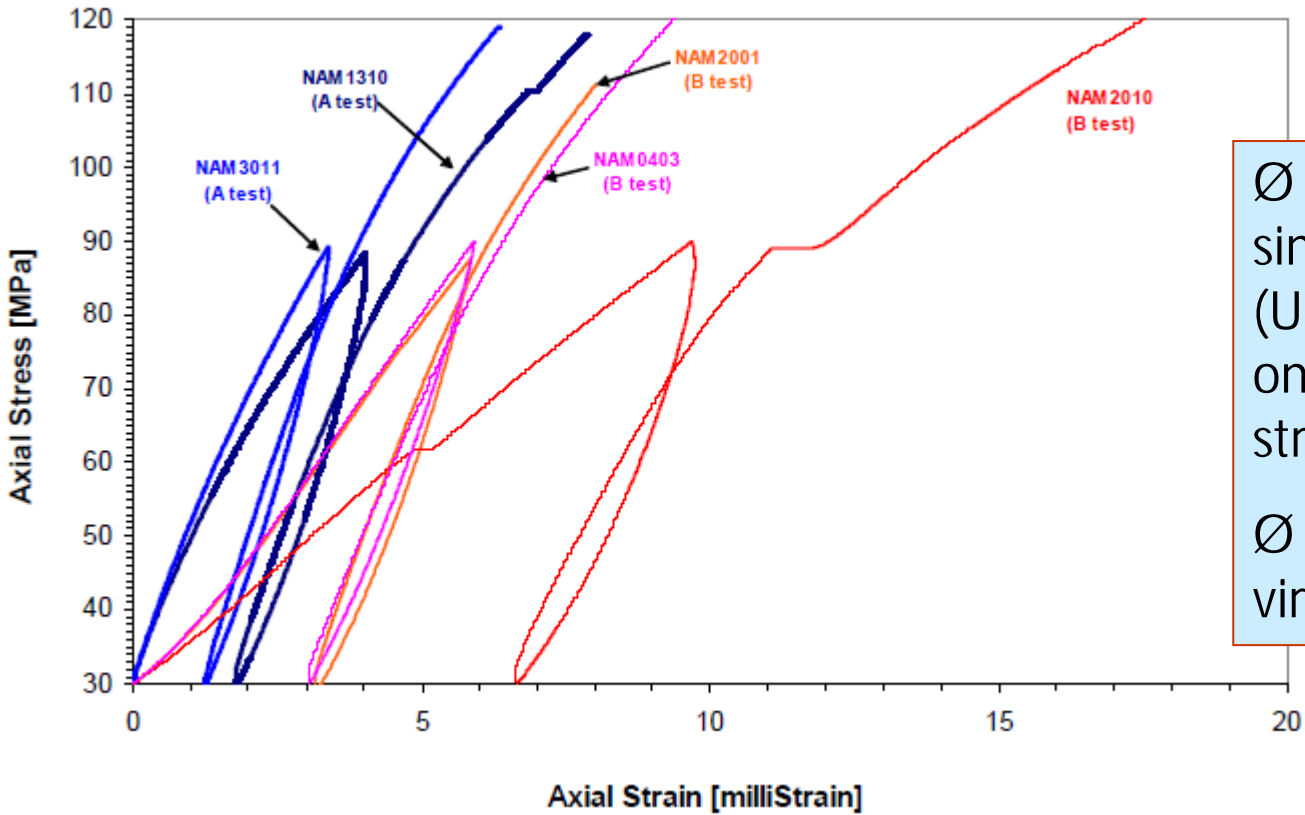
COMPETENT SYNTHETIC ROCK "GRONstone"



- Ø Permanently reduced stiffness of well cemented cored material when reloaded above forming stress: Typical initial stiffness ratio ~ 2
- Ø Apparently similar compaction after the virgin material has reached yield onset: For GRONstone typical 10 MPa (\gg UCS) above the forming ("in situ" stress)

Virgin vs. Simulated Core Compaction

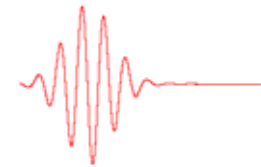
STRONG SYNTHETIC ROCK "EPOXtone"



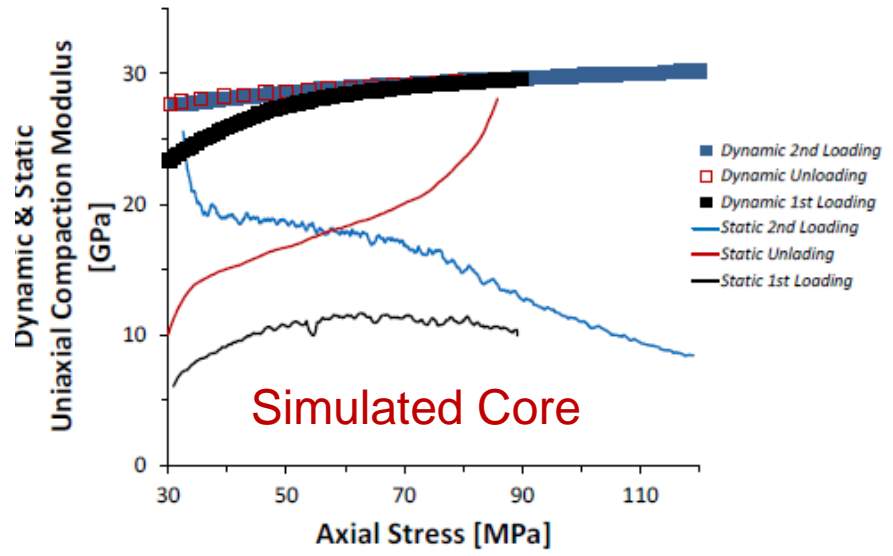
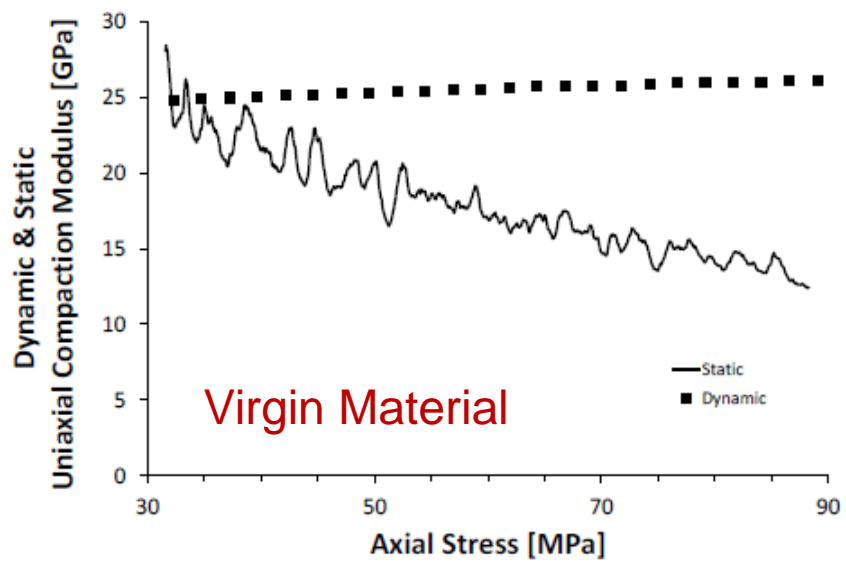
Ø As for GRONstone, but since EPOXtone is stronger (UCS ~ 15 MPa), yield onset occurs at higher stress

Ø Ratio between initial virgin : core stiffness ~ 3-4

Static vs Dynamic Moduli

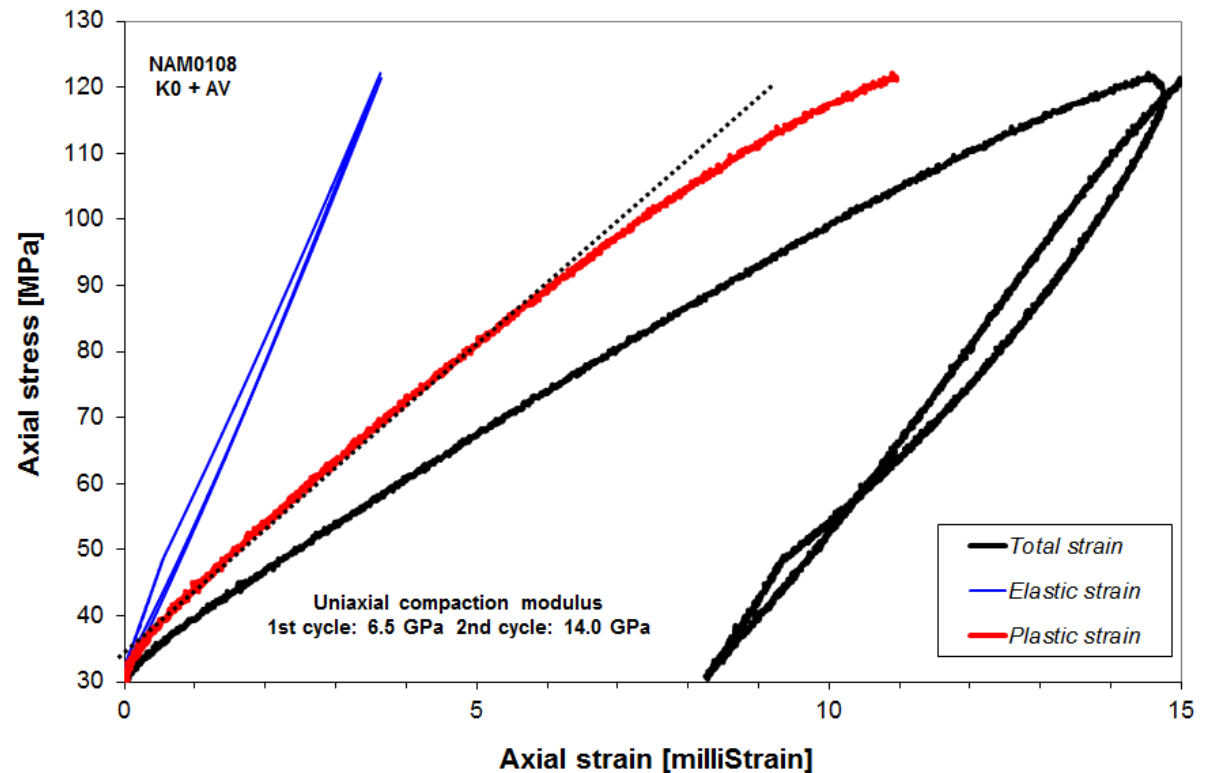


- Ø Epoxy-cemented sandstone, formed at 30 MPa axial & 15 MPa confining stress
- Ø Static = Dynamic Modulus directly after cementation; Undamaged material
- Ø In simulated core, Dynamic > Static modulus, except during stress reversal (unloading + reloading)



NAM Core Compaction

- Field core from NAM tested in uniaxial strain conditions (no pore fluid)
- Note observed nonlinearity (above 80 MPa axial stress) and permanent strains



Why are Static ¹ Dynamic Moduli?

Fluid contribution

Static
drained (normally)

Dynamic
undrained (always)

K_{fr}

«

$K_{fr} + K_f$



Negligible
for gas
saturation

Dispersion

Ultrasonic: $f \sim 1$ MHz

Sonic: $f \sim 10$ kHz

Static: $f \sim 1$ Hz

Plasticity

Static moduli are measured at finite strains and include elastic + plastic deformation; Dynamic moduli are measured at infinitesimal strain and are hence purely elastic.

Static vs. Dynamic Moduli: Strain amplitude effects

Ø Experiments on dry sandstones show that:

Ø In hydrostatic loading (by grain contact plastification, crushing of asperities etc):

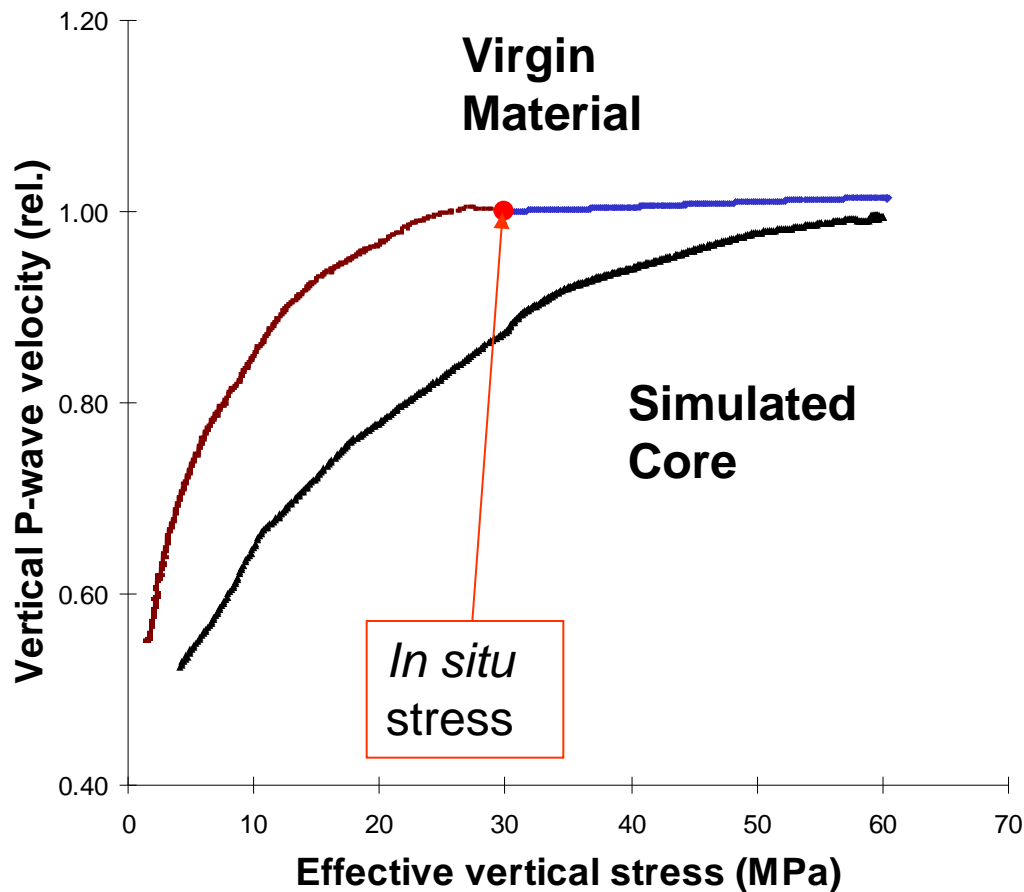
$$K_{stat} = \frac{K_{dyn}}{1 + PK_{dyn}}; P \mu \frac{1}{s + s_0}$$

Ø In triaxial loading (by sliding cracks) :

$$E_{stat} = \frac{E_{dyn}(1 - F)}{1 + P_z E_{dyn}} \quad F \mu e_z - e_r$$

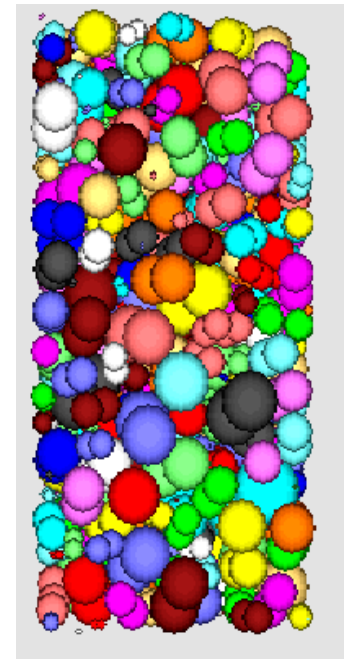
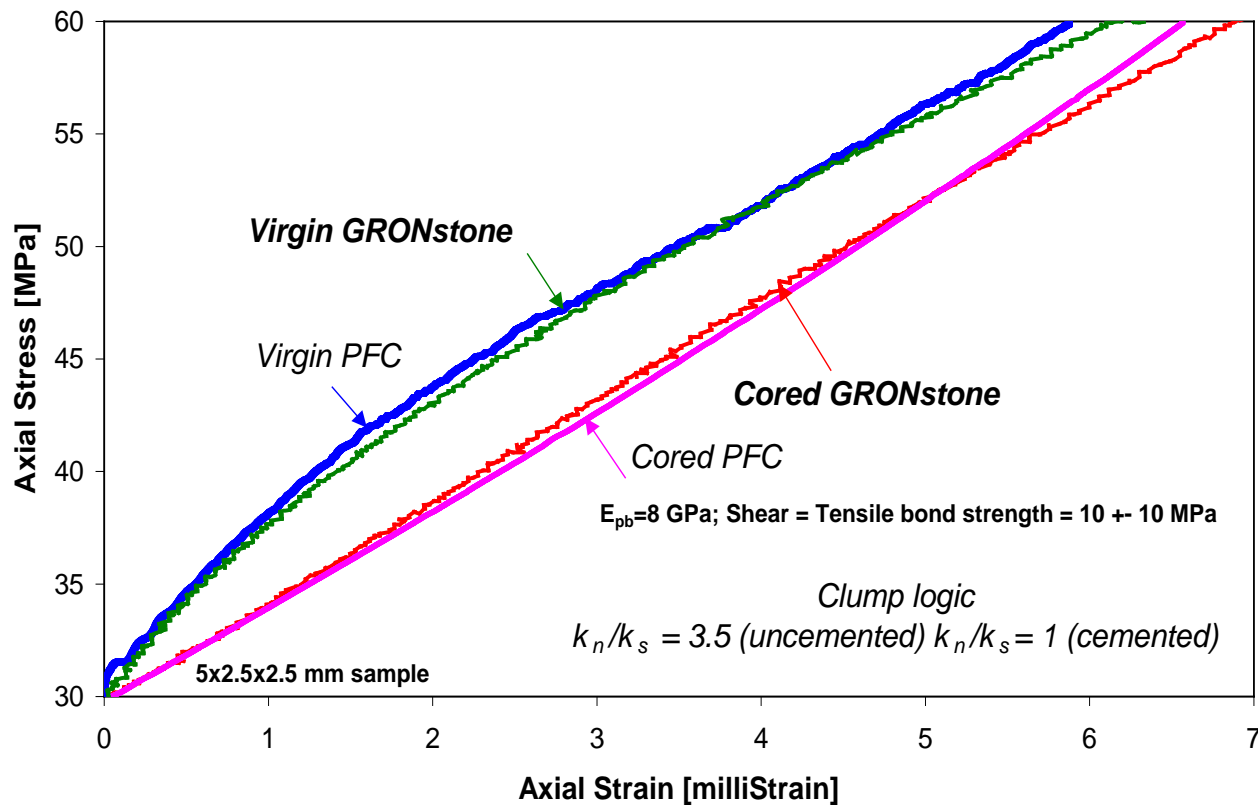
Creep can be modelled within the same framework by making the F-parameter time dependent – viscoplasticity relates to static moduli, viscoelasticity to dynamic

Reservoir Monitoring Aspects: Competent Synthetic Sandstone



- Ø Permanent drop of velocities after coring & reloading to forming stress
- Ø Low stress sensitivity during loading in the virgin material
- Ø Larger stress sensitivity during unloading
- Ø Large stress dependence in the simulated core!

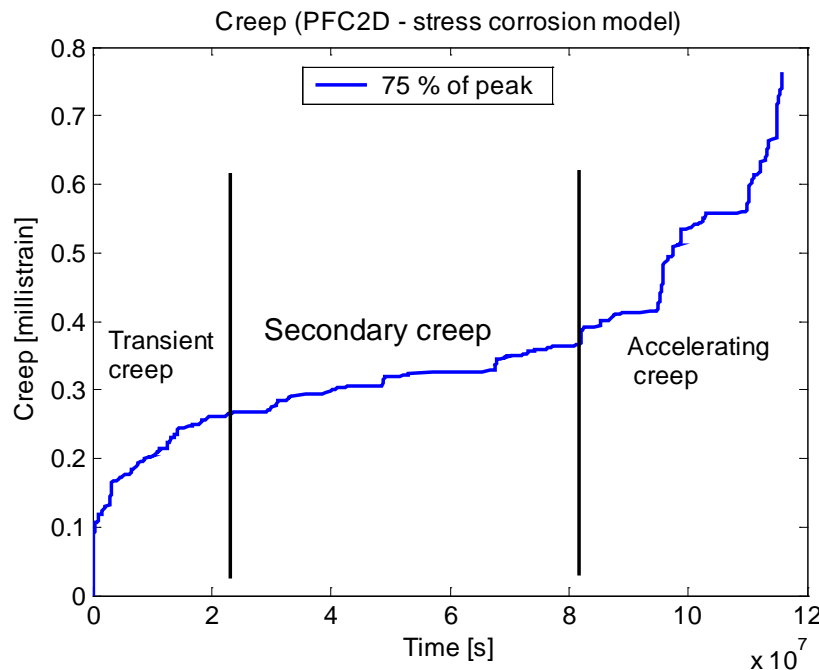
Discrete Particle Modelling: "Best fit" between Laboratory and PFC^{3D} simulations of GRONstone



Looked good....

Time dependent deformation in Discrete Particle Modelling

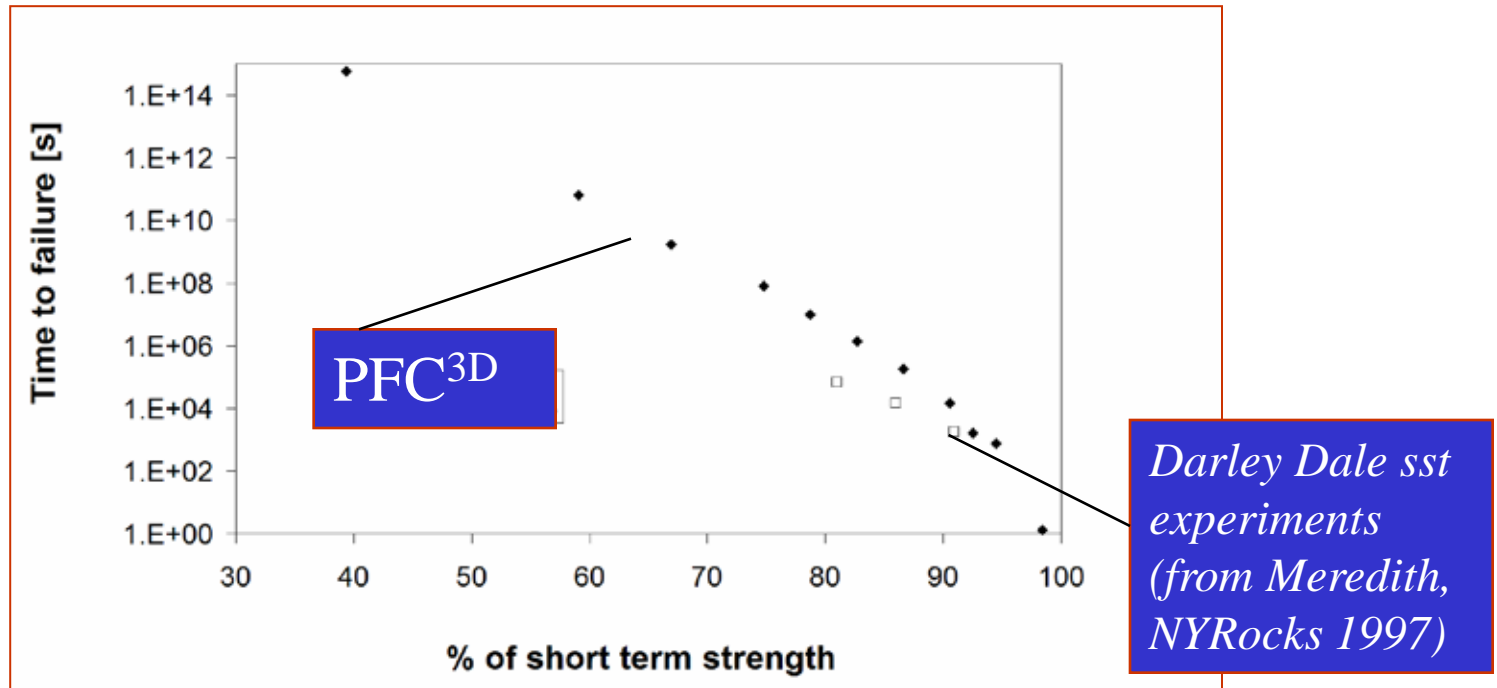
- 6 Creep is implemented to mimick stress-induced corrosion by reducing the parallel bond extent depending on the stress level relative to bond strength at each contact



The model captures the three commonly observed phases of transient, secondary and accelerating (tertiary) creep

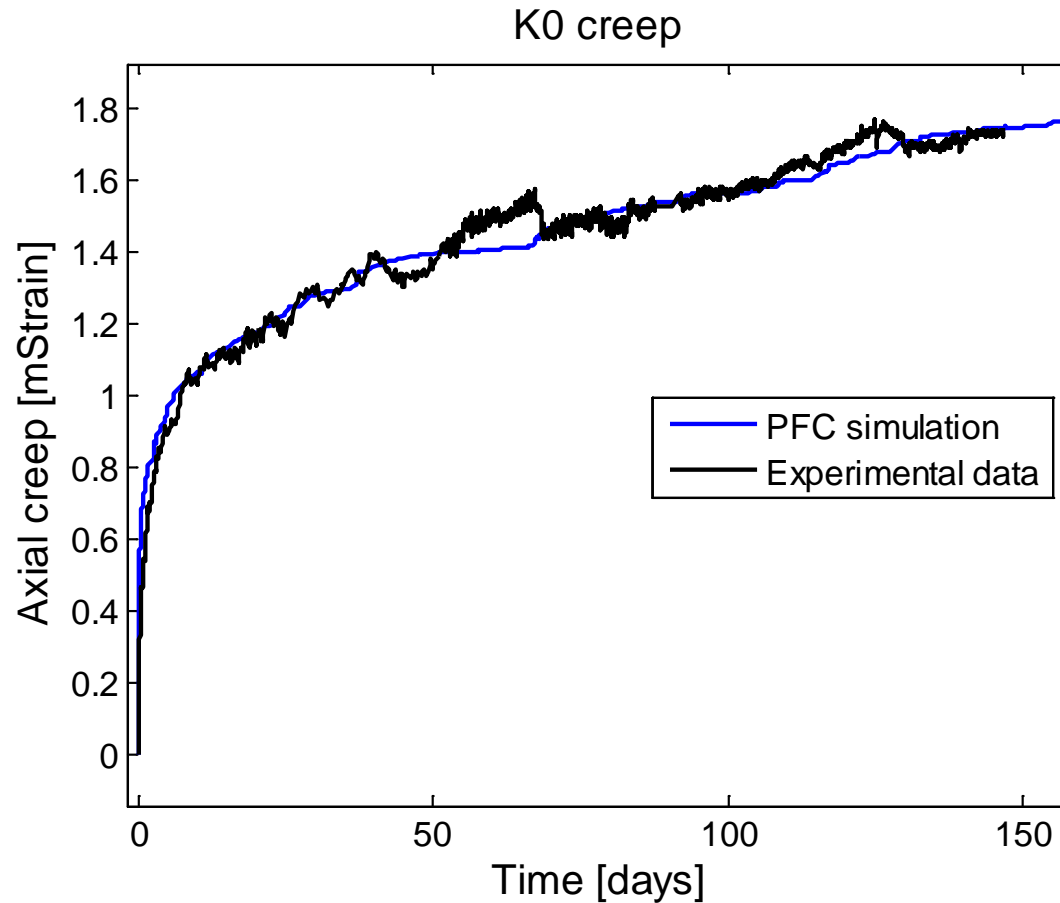
A similar approach has been presented by Potyondy (2005)

Time dependent deformation in Discrete Particle Modelling



- 6 Long-term behaviour may be assessed from short-term simulations
- 6 Challenge: Appropriate calibration of microscopic creep parameters
- 6 Other physical mechanisms may play a vital role over long time scales

Application example: Creep under K_0 conditions



*A tool for
the
future...*

Concluding Remarks

- ∅ Time dependent compaction may be intrinsic (creep, consolidation; within reservoir & overburden) or apparent (stress arching induced, due to onset of plasticity)
- ∅ Rocks deform elasto-plastically – both in the Earth and in the Laboratory
 - ∅ Rock alteration due to stress relief during coring is well and understood, and models for correction of core measured compaction exist
- ∅ Plastic strain evolves as failure is approached, and with it:
Viscoplastic strain
 - ∅ Long term effects may be modelled, but require proper understanding of mechanisms (hard to speed up...)